

HYMO: Process Description

I Water Balance on the Hydrotope/Municipio - Scale

1. Evapotranspiration:

Penman approach for potential ET:

$$PET = \frac{\delta \cdot R_n + c_p \cdot \rho_a \cdot (e_s - e_{act}) / r_a}{(\delta + \gamma) \cdot L_V} \quad (1)$$

where:

PET = potential evapotranspiration

R_n = Net Radiation = f(cloudyness, surface albedo, ...)

$e_s - e_{act}$ = air moisture saturation deficit = f(air temperature, air humidity)

r_a = aerodynamic resistance = f(wind velocity)

non-linear reduction of PET (from the HAPEX-SAHEL study):

$$AET = 0.75 \cdot PET \cdot (1 - \beta_{hum}) \cdot (2.7 - 1.7 \cdot \beta_{hum}) \quad (2)$$

$$\beta_{hum} = \frac{\theta_{FC} - \theta_{act}}{\theta_{FC} - \theta_r} \quad (3)$$

where:

AET = actual evapotranspiration

β_{hum} = reduction coefficient

θ_{act} = actual soil moisture

θ_{FC} = soil moisture at field capacity

θ_r = residual soil moisture

2. Soil Water Balance:

Bucket approach with threshold values $\theta_s, \theta_{FC}, \theta_r$:

$$\theta_r \leq \theta_{act} \leq \theta_s \quad (4)$$

$$\theta_{act}(t + \Delta t) = \theta_{act}(t) + N/\Delta t \quad (5)$$

$$q_o = \begin{cases} 0 & \text{for } \theta_{act}(t) + N/\Delta t \leq \theta_s \\ (\theta_{act}(t) + N/\Delta t) - \theta_s & \text{for } \theta_{act}(t) + N/\Delta t > \theta_s \end{cases} \quad (6)$$

$$q_{gw} = \begin{cases} 0 & \text{for } \theta_{act}(t) + N/\Delta t \leq \theta_{FC} \\ (\theta_{act}(t) + N/\Delta t) - \theta_{FC} & \text{for } \theta_{act}(t) + N/\Delta t > \theta_{FC} \end{cases} \quad (7)$$

$$q_l = f_{subfrac} \cdot q_{gw} \quad (8)$$

$$q_{deep} = (1 - f_{subfrac}) \cdot q_{gw} \quad (9)$$

where:

N	=	precipitation
q_o	=	surface runoff
q_{gw}	=	percolation out of the topsoil ($0 < q_{gw} < k_{sat}$)
k_{sat}	=	saturated conductivity
q_l	=	lateral subsurface flow
q_{deep}	=	deep groundwater recharge
$f_{subfrac}$	=	empirical factor

3. Water budget of distributed ponds (small acudes):

Water balance accounting for inflow, outflow, water use, and evaporation:

$$S_{acude}(t + \Delta t) = S_{acude}(t) + q_{acude}^{in} - q_{acude}^{out} - watuse_{acude} - EV_{acude} \quad (10)$$

where:

S_{acude}	=	water volume in the pond
q_{acude}^{in}	=	surface flow into the pond ($q_{acude}^{in} = f_{intercep} \cdot q_o$)
$f_{intercep}$	=	empirical factor for water collection in the acudes
q_{acude}^{out}	=	outflow from the pond ($0 < q_{acude}^{in} < q_o$)
$watuse_{acude}$	=	water used from the acude (e.g. irrigation)
EV_{acude}	=	open water evaporation from the acude (= f(water surafce))

4. Runoff into the major rivers (only one river stretch per municipio), composed of:

- intercepted surface flow generated in the lowlands
- outflow from lowland acudes
- lateral subsurface flow in the lowlands

$$q_{river} = (1 - (f_{intercep})_l) \cdot (q_o)_l + (q_{acude}^{out})_l + (q_l)_l \quad (11)$$

where:

q_{river}	=	runoff into the municipio's main river
$(q_o)_l$	=	surface runoff in the lowlands
$(f_{intercep})_l$	=	empirical factor for water collection in the acudes (lowlands)
$(q_{acude}^{out})_l$	=	outflow from the ponds (lowlands)
$(q_l)_l$	=	lateral subsurface flow (lowlands)

II Routing in the River System (Scale of Large River Basins)

1. The river network prescribes the water transfer system between all municipalities

\Rightarrow The municipalities are connected by a dendritic river network

2. Routing process through a municipality is approached with a linear response function:

$$Q_j^{out} = \sum_{i=1}^j Q_i^{in} \cdot h_{j-i+1} \quad (12)$$

where:

Q_j^{in} = flow out of the municipality

Q_j^{out} = flow into the municipality

h = response function (hunit-hydrograph type: $h_i > 0; \sum h_i = 1$)

j, i = time interval ($\Delta t = 1d$)

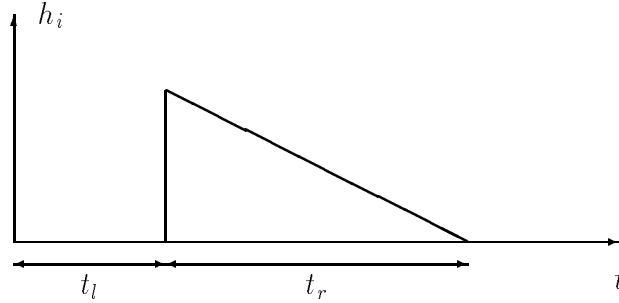


Figure 1: Scheme of the linear response function

where:

t_l = lag time [d]

t_r = retention time [d]

Assumption: $1d \leq t_l + t_r \leq 7d$

$$t_l = \frac{\Delta s}{\bar{v}} \quad (13)$$

$$t_r = \frac{\Delta s}{v_{min}} - \frac{\Delta s}{v_{max}} \quad (14)$$

where:

Δs = channel length

t_l = lag time [d]

t_r = retention time [d]

3. Flow velocity in the river

\Rightarrow The flow velocity in the river by Manning equation:

$$v = k_{St} \cdot \sqrt{S_0} \cdot \left(\frac{h \cdot B}{2h + B} \right)^{\frac{2}{3}} \quad (15)$$

where:

$$\begin{aligned} k_{St} &= \text{roughness coefficient (Strickler coefficient } [m^{1/3}/s]) \\ S_0 &= \text{channel slope } [-] \\ B &= \text{channel width [m]} \\ h &= \text{flow depth [m]} \end{aligned}$$

\Rightarrow Flow velocity: $f(k_{St}, S_0, h, \text{ river cross section})$

Assumption:

$$\begin{aligned} v_{max} &= v(h = h_{max}) \\ \bar{v} &= v(h = \frac{2}{3}h_{max}) \\ v_{min} &= v(h = \frac{1}{10}h_{max}) \end{aligned}$$

4. Estimation about the cross section of the river

\Rightarrow approached given in the CREAMS–SWAT/GRASS–interface WILLIAMS, 19??)

$$H = 0.13 \cdot (A_{acc}^E)^{0.4} \quad (16)$$

$$B = 1.29 \cdot (A_{acc}^E)^{0.6} \quad (17)$$

where:

$$\begin{aligned} H &= \text{channel depth [m]} & 0.25\text{m} < H < 20\text{m} \\ B &= \text{channel width [m]} & 0.5\text{m} < B < 500\text{m} \\ A_{acc}^E &= \text{accumulated catchment area [km}^2] \end{aligned}$$

III Water Storage in Large Reservoirs

1. Water balance equation for the reservoir

$$S(t + \Delta t) = S(t) + Q_{in} \cdot \Delta t - Q_{out} \cdot \Delta t + N \cdot \Delta t \cdot A^R - EV \cdot \Delta t \cdot A^R - PERC \cdot \Delta t \quad (18)$$

where:

S	=	actual water volume stored in the reservoir ($0 \leq S \leq V_{max}$)
V_{max}	=	maximum water volume stored in the reservoir
Q_{in}	=	river flow into the reservoir
Q_{out}	=	outflow from the reservoir
N	=	precipitation
EV	=	lake evaporation (=potential evapotranspiration)
$PERC$	=	percolation
A^R	=	water surface of the reservoir

2. Outflow from the reservoir is composed of controlled outflow and excess outflow:

$$Q_{out} = Q_{contr} + Q_{spill} \quad (19)$$

where:

Q_{contr}	=	controlled, managed outflow from the reservoir (<u>water use</u>)
Q_{spill}	=	excess, unmanaged outflow (<u>over the spillway</u>)

first approach: $A^R = S/50 \text{ m}$ (S in m^2)
 $N \cdot \Delta t \cdot A^R$: neglected
 $PERC \cdot \Delta t$: neglected

HYMO: Model Variables

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1. Evapotranspiration:

- Net Radiation R_N
- (cloudyness, surface albedo, ...)
- air moisture saturation deficit ($e_s - e_a$)
- air temperature, air humidity
- wind velocity
- θ_{act} (actual soil moisture)
- θ_{FC} (soil moisture at field capacity)
- θ_r (residual soil moisture)

2. Soil Water Balance:

- N (precipitation)
- q_o (surface runoff)
- q_{gw} (percolation out of the topsoil)
- q_l (lateral subsurface flow)
- q_{deep} (deep groundwater recharge)

3. Water budget of distributed ponds (small acudes):

- q_{acude}^{in} (surface flow into the pond)
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II Routing in the River System (Scale of Large River Basins)

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- Q_j^{in} (flow out of the municipio)
- Q_j^{out} (flow into the municipio)

III Water Storage in Large Reservoirs

1. Water balance equation for the reservoir

- S (actual water volume stored in the reservoir)
- Q_{in} (river flow into the reservoir)
- Q_{out} (outflow from the reservoir)
- N (precipitation)
- EV (lake evaporation (=potential evapotranspiration))
- $PERC$ (percolation)
- A^R (water surface of the reservoir)
- Q_{contr} (controlled, managed outflow from the reservoir (water use))
- Q_{spill} (excess, unmanaged outflow (over the spillway))

HYMO: Model Parameters

I Water Balance on the Hydrotope/Municipio - Scale

1. Evapotranspiration:

- β_{hum} (reduction coefficient)
- θ_{FC} (soil moisture at field capacity)
- θ_r (residual soil moisture)

2. Soil Water Balance:

- k_{sat} (saturated conductivity)
- $f_{subfrac}$ (empirical factor)

3. Water budget of distributed ponds (small acudes):

- S_{acude} (water volume in the pond)
- $f_{intercep}$ (empirical factor for water collection in the acudes)

4. Runoff into the major rivers (only one river stretch per municipio), composed of:

- $(f_{intercep})_l$ (empirical factor for water collection in the acudes (lowlands))

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1. The river network prescribes the water transfer system between all municipios
Routing process through a municipio is approached with a linear response function:

- k_{St} (roughness coefficient (Strickler coefficient [$m^{1/3}/s$]))
- S_0 (channel slope [-])
- B (channel width [m])
- H (channel depth [m])
- A_{acc}^E (accumulated catchment area [km^2])

III Water Storage in Large Reservoirs

1. Water balance equation for the reservoir

- V_{max} (maximum water volume stored in the reservoir)
- A^R (water surface of the reservoir)